150sno01.dwg

STRUCTURAL NOTES

These notes shall govern unless noted otherwise on the drawings. Refer to written specifications for further requirements and amplification of these notes.

BUILDING CODE

Design fabrication & construction	IBC 2003 with ASCE 7-02

DESIGN LOADS

Gravity - Superimposed Dead Loads:

ROOF	
Membrane & Rigid Insulation	4.0 PSF
3" x 20ga type N roof deck	3.0 PSF
28 LH Joists @ 10'-0" O.C.	3.0 PSF
Primary framing (X-Hall)	13.0 PSF
MEP, ceiling, etc	5.0 PSF
Rigging (X-Hall only)	20.0 PSF

Gravity - Superimposed Live Loads:

Floor - Office	50 PSF
Floor - parking	50 PSF
Floor - X-Hall	300 PSF
Stairs/Corridor	100 PSF
Mechanical room floor	125 PSF

Gravity - Snow Loads:

Snow Importance Factor	1.10
Ground Snow Load (Pg)	36 PSF
Roof Snow Load (Pf)	30 PSF (Min. uniform load)
Drift as calculated per ASCE 7-02	

Wind Loads:

Main Wind-Force Resisting System (MWFRS)	
Fastest Mile Wind Speed	70 MPH
3 Second Gust Wind Speed	85 MPH
Importance Factor (Iw)	1.15
Exposure Category	B
Internal Pressure Coefficient	+/- .18
Design Wind Pressure	Per calculations
Roof Uplift (Net)	10 psf
Building Components and Cladding	Per ASCE 7-02

Seismic Loads:

Structural design requirements per IBC 2003:	
Seismic Importance Factor (Ie)	1.25
Spectral Response Acceleration (Ss)	0.315
Spectral Response Acceleration (S1)	0.092
Site Class for Soil Profile	A
Seismic Use Group	III
Seismic Design Category	B
Spectral Response Coefficient (Sds)	0.17
Spectral Response Coefficient (Sd1)	0.05
Design Base Shear	3270 kips
Analysis Procedure	Equivalent Force
Basic Seismic-Force-Resisting System:	IBC 1617.6.1.2
Above X-Hall Floor	
Cantilevered Column System	V = 285 kips
Response Modification Coefficient (R)	2.5
System Overstrength Factor	2
Deflection Amplification Factor	2.5
Below X-Hall Floor	
Ordinary Steel Concentric Braced Frames	V = 2985 kips
Response Modification Coefficient (R)	5
System Overstrength Factor	2
Deflection Amplification Factor	4.5

Earth Pressure

Equivalent fluid pressure method	
Passive Pressure	300 PCF
Active Pressure	40 PCF
At Rest Pressure	65 PCF
Sliding Coefficient (concrete on rock)	0.45

150sno02.dwg

CONCRETE

Concrete Mix Requirements Per ACI 301-XX, section 1.6 w/test submittals to Engineer/Architect or use chart below.

Type	Use	28 Day Strength PSI f'c	Max. Aggregate Size	Slump * Range (inches)	% Air
A	Sitework & Footings	3000	3/4"	3-5	5-7
B	Exposed to Weather (incl. Retaining Wall Footings, Walls & Grade Beams)	4000	3/4" 1/2"	2-4	5-7
C	Lean Concrete	2000	---	---	---
D	Stair pan topping & HCS cell grout(t=3"or less)	3000	3/8"	2-4	2 max
E	Columns	5000	3/4"	2-4	---
F	All Others	4000	3/4" 1/2"	2-4	2 max
G	Truck Parking Topping Slab	3000	3/4"	2-4**	5-7
H	Interior Slab on Grade (5 sack minimum mix)	2500	3/4"	2-4	2 max
J	Drilled Piers See Spec Section 02459				
	Note: Types A, C, and E concrete shall have not less than 12% fly ash by weight of cementitious materials.				

* Slump to be determined before polyfiber or superplasticizer is added (if required). Slump test not required for interior slab on grade.

** Mix to contain Fibermesh InForce 3 Filibrated Polypropylene Fibers, 3/4" lg., 3 lbs/Cu Yd. Add superplastizer at site to adjust slump for workability.

Concrete Mix Proportions Per ACI 301, Section 4.1

Conduits embedded within slab, wall or beam: (conduits to be placed between rebar mats where double mats occur.) Diameter smaller than 1/4 x member thickness. Spacing greater than 4 x conduit diameter.

Camber concrete forms: As shown plus deflection due to weight of wet concrete

Additives

to exterior slabs:	Polyfiber Reinforcing (CFP) unless noted otherwise on Civil drawings
to unreinforced topping slabs:	Polyfiber Reinforcing and Superplasticizer
to reinforced topping slabs:	Superplasticizer (6" to 8" slump)
to stair pans	Polyfiber Reinforcing (CFP)

150sno03.dwg

REINFORCING

	Deformed Bars	ASTM A615 Grade 60
	Weldable Bars	ASTM A706
	Tie Wire	Double annealed 16 GA. iron wire
	Field Bends	Cold bend bars noted on dwgs to radius as per CRSI. Bend bars 1 time only.
	CMU Vertical Rebar Positioners	4'-0" o.c. max.
	Hooked Reinforcing	Engage with min. #4 continuous (2'-0" long min.)
Concrete Cover	Cast against earth	3"
	Formed surfaces exposed to earth or weather	1½" #5 or smaller 2" #6 or larger
	Not exposed to earth or weather	¾" #11 or smaller
	Beam or column stirrups & ties	1½"

Wall Trim Bars May be omitted when the opening dimensions are less than the rebar spacing, for openings less than 12" x 12" relocate interrupted rebar along one side of the opening and add an identical bar on the other side. Alternatively, it may be sprung around the opening with no additional rebar required if it lies within 3" of the edge. For larger openings, see specific typical detail.

Shop Drawings Wall elevations are required for all walls on 24"x36" sht. min. and drawn to ¼" min. scale. Show all openings larger than 12" in any dimension.

Before submitting shop drawings for Structural Review, Mechanical & Electrical Contractors to indicate size & location of all penetrations.

Welding

ASTM A706	Subject to approval of Architect/Engineer
ASTM A615	Not allowed
Welders	AWS D1.4 preqaulified for all sizes & positions required.

Bar Supports

In contact with earth	Precast concrete blocks 4000 PSI strength @ 2'-0" o.c. max. wire tied to alternating bar intersections.
All others	Class 1 plastic tip @ 2'-0"o.c. max. wire tied to alternating bar intersections.

150sno12.dwg

REMODEL WORK

Contractor shall verify all existing conditions and inform Engineer of any differences prior to and during any demolition work. No removal or alteration of the existing structures will be allowed until the Engineer (or his field representative) has reviewed:

- Sequence of work
- Method of shoring
- Extent and method of demolition

Saw cut existing concrete and masonry walls at least 1" deep around all openings prior to removal. Over cutting not allowed.

150sno23.dwg

EXISTING CONDITIONS 1

1. DISCREPANCIES: VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AND CONTACT THE ENGINEER OF RECORD IF ANY DISCREPANCIES ARE FOUND BEFORE PROCEEDING. NO REMOVAL OR ALTERATION OF THE EXISTING STRUCTURE WILL BE ALLOWED UNTIL THE ENGINEER (OR HIS REPRESENTATIVE) HAS REVIEWED SEQUENCE OF WORK, METHOD OF SHORING, AND EXTENT & METHOD OF DEMOLITION.
2. FULLY COORDINATE WITH OWNER TO IDENTIFY LOCATION AND EXTENT OF WALL PARTITIONS, CEILINGS, AND/OR FLOORS TO BE REMOVED.
3. MAKE EVERY EFFORT TO MINIMIZE DISRUPTION TO OWNER'S OPERATION AND TO PROVIDE PATRON, FACILITY STAFF AND WORKERS SAFETY.
4. COORDINATE WITH THE OWNER'S REPRESENTATIVE ON APPROVAL FOR EXCESSIVE NOISE AND VIBRATION.
5. PROVIDE PROTECTION FOR ALL EXISTING BUILDING MATERIALS AND EQUIPMENT TO REMAIN FROM DAMAGE DUE TO DEMOLITION OR CONSTRUCTION OPERATIONS PERFORMED UNDER THIS CONTRACT.
6. REMOVE ALL DEMOLITION MATERIALS FROM THE SITE UNLESS OTHERWISE NOTED AND DISPOSE OF IT IN A LEGAL MANNER.
7. REPAIR OR REPLACE ANY WALLS, FLOORS, CEILINGS OR EQUIPMENT DAMAGED AS A RESULT OF DEMOLITION OR CONSTRUCTION TO MATCH EXISTING FINISH AND CONDITION.
8. MATERIALS AND FINISHES FOR NEW WALLS, FLOORS, AND CEILINGS TO MATCH SURROUNDING SURFACES UNLESS NOTED OTHERWISE. MAINTAIN RATING OR REPLACE WITH CONSTRUCTION OF THE SAME RATING ALL EXISTING FIRE AND SMOKE RATED CONSTRUCTION.
9. VERIFY AND MAINTAIN THE LOCATION OF EXISTING PLUMBING, POWER, COMMUNICATION AND DATA CABLES SO AS NOT TO INADVERTENTLY INTERRUPT THE CONTINUITY OF THEIR SERVICES. UTILITIES ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS.
10. LEAVE ALL AREAS OF WORK BROOM AND DUST CLEAN ON HARD SURFACES AND VACUUM CLEANED ON CARPETED SURFACES.
11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SEQUENCES OF CONSTRUCTION FOR PROVIDING ALL TEMPORARY GUYS, BRACING, AND OTHER SUPPORTS AS NEEDED TO SAFELY RESIST ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING LOADS ERECTION EQUIPMENT AND OPERATIONS, AND WIND OR SEISMIC FORCES COMPARABLE IN INTENSITY FOR WHICH THE STRUCTURE WAS DESIGNED.
12. MEET THE REQUIREMENTS OF ALL APPLICABLE CODES AND ORDINANCES, INCLUDING THE PROVISIONS OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES, NINTH EDITION FOR ALL ERECTION AND CONSTRUCTION PROCEDURES.
13. IF HAZARDOUS MATERIALS ARE ENCOUNTERED STOP WORK IMMEDIATELY IN THAT AREA OF WORK AND NOTIFY THE OWNER.
14. REMOVE AND REPLACE FINISHED SURFACES AS REQUIRED TO PERFORM STRUCTURAL WORK.

150sno06.dwg

CONCRETE MASONRY UNITS

Specification	ASTM C90-93	Grade N Type I
	Unit weight	115 pcf min.
Compressive Strengths at 28 Days	Individual Units	1900 psi (net area)
	Mortar	Type S (field test) [1800 psi]
	Grout	2000 psi
Design Strengths	(Unit Strength Method) per IBC 2105.2.2.1	f'm = 1500 psi
Bracing of Walls	All walls	U.N.O. Extend to struct. above
	Non-load Bearing walls	Shown "NLB" on drawings
	Partial ht. Walls (Extend above ceiling)	Shown "PHW" on drawings
	Bracing for NLB & PHW	4'-0"o.c. U.N.O. per Typical Detail.
	Bracing not required	If cross walls occur less than: 16'-0" for 12" CMU 14'-0" for 8" CMU 10'-0" for 6" CMU
Anchors	Adhesive Anchors	Install in fully grouted cells
	Embedded Anchors#	Install in fully grouted cells
	Expansion Anchors	Install in fully grouted cells
	Screw Anchors	Install in fully grouted cells
# 1" min grout between anchor washer and masonry		
Grout Pours	5'-0" High max	Mechanically vibrate twice per ACI 530.1 article 3.5
Conduits	Embedded in CMU	1½" clear from all parallel reinforcing
CMU Walls Not Dimensioned On Plans	(See Architectural dwgs)	

150sno07.dwg

STRUCTURAL STEEL

Design Guide		"AISC Manual of Steel Construction Load & Resistance Factor Design" Third Edition or ASD Ninth Edition
Materials	Wide Flange Shapes	ASTM A992 fy=50KSI
	Bars, Plates, Shapes	ASTM A36 fy=36KSI
	Tubes	ASTM A500 GR B fy=46KSI
	Pipe	ASTM A53 Type E fy=35KSI
	Bolts	A325, load indicating bolts
	Welding	AWS D1.1
	Anchor bolts	A36 or A307 w/ washer
	Steel Joist	Per Steel Joist Institute specs.
	Cont. threaded rod	A36 w/washer ea. connection
	Erection bolts	A307 (snug tight.)
Fabrication	Bolted Shear Connections	N=Bearing ¾" dia bolts
	Short slotted holes	U.N.O. Place hardened washers between nuts & slotted holes.
	Open web steel joists & girders	Steel Joist Institute Standard Specification & Current NER Report
	Welds	E70XX UNO. (T-40 not allowed)
	Welders	AWS D1.1 preqaulified for all sizes & positions required.
Placement	Tube Steel	Place seam to non-visible side
	Field Bolted or Welded Connections	Complete Connection before releasing Crane cable

150sno14.dwg

SPECIAL INSPECTION

Per IBC 2003 Chapter 17 — Arrange Schedule to suit Special Inspector

		Visual Inspection (1) Non Continuous	Continuous	Test
Concrete	Slab-on-Grade	--	--	(7)
	Formwork	100%	--	--
	Reinforcing & CIP Anchors before form enclosure	100%	--	--
	Concrete Strength & Slump	--	--	yes
	Placing	--	100%	--
	"Drilled in" Anchors	95%	5%	(3)
Welding	Field welds 5/16" or less	25%	--	--
	All other welds greater than 5/16" & fp's	--	100%	(2)
	Floor and roof deck welding, & button punching.	100%	--	--
	Field welded studs	100%	--	2%
High Strength Bolting	Tension & shear connections	100%	5%	--
	Pretested connection required prior to bolting	--	--	yes(3)
CMU Non-continuous special inspection Level 1 - IBC Table 1704.5.1 (B)				
Pier Foundations	Bearing verification	100%	-	
	Reinforcing placement	100%	-	
	Concrete placement	-	100%	
Fireproofing Struct. Steel	Spray applied (Spec. Section 07810)	--	100%	

(1) Visual Inspection: Non-Continuous = Percent of product inspected after installation.
Continuous = Percent of product inspected during installation.

(2) Weld Testing: Per AWS D1.1 (zone 3 & 4 only) — Ultrasonic or radiographic

(3) Procedure Verification of First 5%

(4) Certification letter required

(5) Site mixed - conform to UBC Table 2109.3.2
Preblended - certification letter required

(6) 3 lab tests before construction
3 field tests per 5000 s.f. during construction

(7) Testing per 03300 surface finish

SUBMITTALS TO SPECIAL INSPECTOR

		Certificate
Welding	Structural steel	AWS D1.1 Preqaulification
	Light gage steel	AWS D1.3 Preqaulification
	Reinforcing	AWS D1.4 Preqaulification
	All Welders	WABO Certified
Bolting	High strength	Mfr. inspection cert.
CMU Walls	Block, Grout, Mortar	Mfr. letter of cert.
	Quality control per IBC sect. 2105.2.2.1 w/results for review & approval prior to construction	
Adhesive Anchors	Epoxy adhesive	Mfr. installation instructions
Concrete	Trip tickets	All materials, water/cement ratio in each truck
Slab on Grade	In place W/C Ratio	(Submit to Struct. Engineer)

Inspector to review submittals before installation begins. Inspector must inspect job from approved & stamped design & shop drawings, including all clarifications, both written & in drawing form.

150sno04.dwg

DRAWING NOTES

Penetration of beams, joists or columns	Not allowed unless approved by the architect or noted on the structural dwgs
Mechanical equipment and housekeeping pads	For location and sizes see Division #15 & #16
Openings	Not all openings are shown on structural plans, coord. all drawings
Repetitive Fasteners	Place not more than 6" from the end of member
Powder Driven Fasteners- 0.177"x 1¼" lg. U.N.O.	In Concrete- 4" min. o.c. 3" edge distance In Steel- 1½" min. o.c. 1½" edge distance
Concrete Screws- ¼"x1¾" lg.U.N.O.	3" min. o.c. 2½" min. edge distance
Grids	Unless noted otherwise, Aligned beams,columns, walls & foundations shall have a common centerline (Grid)
Chamfer	¾" on all exposed conc. edges
Reqyest for specific approval	Flag on shop drawing "Architect Approve"
Shop drawings	Construction shall not occur until approved shop drawings are on site.

9sno15.dwg

ABBREVIATIONS

AB	Anchor Bolt	LLH	Long Leg Horizontal
ADD'L	Additional	LLV	Long Leg Vertical
ADH	Adhesive	LNDG	Landing
AESS	Architecturally Exposed Structural Steel	LSH	Long Slotted Hole
		LSL	Laminated Stud Lumber
AFF	Above Finished Floor	LVL	Laminated Veneer Lumber
ALT	Alternate		
ARCH	Architect		
ATR	All Threaded Rebar	MAT	Material
AWS	American Welding Society	MAX	Maximum
		MB	Machine Bolt (A-307)
		MCR	Modified Chloroprene Rubber
BCX	Bottom Chord Extension		
BTWN	Between	MEZZ	Mezzanine
BLDG	Building	MFR.	Manufacturer
BM	Beam	MLB	Microlam Beam
BOT	Bottom	MIN	Minimum
BOD	Bottom of Deck	MO	Masonry Opening
BP	Button Punch		
BRG	Bearing	N-GROUT	Non Shrink Grout
		NIC	Not In Contract
C	Camber	NLB	Non Load Bearing
C-GROUT	Coarse Grout	NOM	Nominal
CFP	Polyfiber (See Specs.)	NS	Near Side
CIP	Cast In Place	NTS	Not To Scale
CJ	Construction Joint		
CJP	Complete Joint Penetration	OC	On Center
		OF	Outside Face
℄	Center Line	OH	Oversized Hole
CLR	Clear	OPG	Opening
CMU	Concrete Masonry Unit	OPP	Opposite
COL	Column		
CONC	Concrete	PL	Plate
CONN	Connection	PC	Precast Concrete
CONT	Continuous	PDF	Power Driven Fastener
CTR	Centered	PHW	Partial Height Wall
		PLYWD	Plywood
DBA	Deformed Bar Anchor	PS	Prestressed
DBL	Double	PTFE	Polytetrafluoroethylene
DET	Detail	PVC	Poly Vinyl Chloride
DIA ⌀	Diameter		
DIM	Dimension	R	Radius
DO	Ditto	RC	Reinforced Concrete
DP	Deep	REINF	Reinforcing
DWG, DWGS	Drawing, Drawings	REQ'D	Required
DWL, DWLS	Dowel, Dowels	RETG	Retaining
		RFRE	Random Fiber Reinforced Elastomeric
EA	Each		
EF	Each Face	RTU	ROOF TOP UNIT
EW	Each Way		
EL	Elevation	SEC	Security
ELEV	Elevator	SCH	Schedule
EXP	Expansion	SECT	Section
EXIST	Existing	SHTG	Sheathing
EXT	Exterior	SIM	Similar
		SPA	Spacing
FAB	Fabricate	SQ #	Square
FB	Flat Bar	STD	Standard
FCJ	Floor Control Joint	STG'D	Staggered
FDN	Foundation	STL	Steel
F-GROUT	Fine Grout	STRUCT	Structural
FIN	Finish	SYM	Symmetrical
FLG	Flange	SOG	Slab-On-Grade
FLI	Ferrule Loop Insert	SS	Stainless Steel
FLR	Floor	SSH	Short Slotted Hole
FOC	Face of Concrete		
FOM	Face of Masonry	T & B	Top and Bottom
FOS	Face of Stud	TCX	Top Chord Extension
FRC	Flanged Rebar Coupler	THD	Thread, Threaded
FS	Far Side	TOC	Top of Concrete
FT	Feet	TOF	Top of Footing
FTG	Footing	TOP	Top of Parapet
		TOS	Top of Steel
GA	Gage	TPG	Topping
GALV	Galvanized	TRA	Threaded Rod in Adhesive anchor
GLB	Glue Laminated Beam		
GR	Grade	TRANS	Transverse
GNB	Gypsum Wallboard	TYP	Typical
HCS	Hollow Core Slab		
HORIZ, H	Horizontal	UBC	Uniform Building Code
HSB	High Strength Bolt	UNO	Unless Noted Otherwise
HT	Height		
		VERT, V	Vertical
ICMU	Insulated CMU	VMS	Vertical Movement System
IF	Inside Face		
IJ	Isolation Joint		
INFO	Information	W/	With
INT	Interior	WP	Work Point
		WS	Weld Stud
JST	Joist	WWF	Welded Wire Fabric
JT, JTS	Joint, Joints	WCJ	Wall Control Joint
LIB	Load Indicator Bolt	X-STR	Extra Strong
LGT	Longitudinal	XX-STR	Double Extra Strong

150sno21.dwg

DEFERRED SUBMITTAL LIST

STRUCTURAL

01400	SPECIAL INSPECTION
03200	CONCRETE & MASONRY REINFORCING
03300	CONCRETE MIX DESIGN
03450	PRECAST CONCRETE
03600	GROUT
04065	MASONRY MORTAR AND GROUT
04820	CONCRETE MASONRY UNITS
04813	MASONRY VENEER TIES
05120	STRUCTURAL STEEL
05210	STEEL JOISTS
05311	STEEL COMPOSITE DECK
05312	STEEL ROOF DECK
05500	METAL FABRICATIONS
05510	METAL STAIRS
05520	METAL HANDRAILS AND RAILINGS

150sno05.dwg

FOUNDATIONS

Bearing Foundations	Allowable net bearing pressure:	Rock 40 ksf Soil 1.5 ksf
	Maximum slope between adjacent footings:	1 Horizontal to 1 Vertical in rock
	Basis of design:	Geotechnical Report prepared by: Geo Engineers Dated September 25,2002
Pier Foundations	Minimum pier diameter:	3 feet
	Minimum penetration into weathered rock:	to sound hard basalt
	Allowable pier capacity:	40 ksf in end bearing

150sno16.dwg

LEGEND

- Column in plan - Stops at this Level
- Column in plan - Continues above this level
- Hold Down
- Existing Concrete in plan or section
- Cast-in-Place Concrete in plan or section
- Precast Concrete in plan or section
- CMU in plan or section
- Structural Stud Wall
- Lintel Beam in Plan
- Natural Grade/Earth
- Structural Fill
- Compacted Gravel
- Undisturbed Rock
- Capillary Break
- Timber Connector (Simpson Strongtie Designation)
- Structural Stud Shear Wall
- Column/Wall Footing
- Stepped Footing
- Match Line
- Floor Control Joint (FCJ)
- Existing Building Line
- 100'-0" T.O.F — Top of Footing Elevation
- EL 100.00' — Top of Slab Elevation
- DEPTH — Depth of Depression
- Slab Depression
- Extent of Depression in Plan
- 20LH 280/140 — Open Web Steel Joist Designation [Depth] [Type] [Total Load]/[Live Load] (PLF) (PLF)
- W12X26 C= N= — Denotes Beam Size; Denotes Camber & Number of Headed Weld Studs (where they occur)
- Denotes Flush Frame Connection Joist or Girder
- Direction of Framing
- Extent of Framing
- TS 4x4x¼ — Column size Designator
- Column Footing Designator

DETAIL REFERENCE SYMBOLS:

- 1 / S2.2 — Denotes Section Number & Direction of View; Denotes Sheet Detailed on
- 1 / S2.2 — Denotes Section Number; Denotes sheet that Section is cut on.
- A — Reference for Portions of details

150sno08.dwg

LIGHT GAGE STEEL

Design & Fabrication	AISI "Specification for Cold Rolled Steel"
Structural Metal Studs	Yield Strength fy=33ksi galvanized (Structural walls are those shown on structural drawings and include exterior walls, loadbearing walls, shear walls, fascias, soffits, and interior non-bearing walls as shown
Screws	#12-24 TEK U.N.O. #10 Where designated to be #10-24.
Roof Deck	ASTM A653 galvanized
Composite Deck	ASTM A653 galvanized
Welders	AWS D1.3, preqaulified for all sizes & positions required.
Welds	E60 xx (SMAW)
Typical Exterior Wall Studs	600S162-43 @ 16" O.C. max vert span = 18'-0" u.n.o.
Typical Interior Wall Studs	600S162-33 @ 16" O.C. max vert span = 26'-0" for over 26'-0" use exterior stud size and spacing. Span height is based on full sheathing each face of stud.

JOHN D. CUDDY — STATE OF WASHINGTON — 19444 — REGISTERED PROFESSIONAL ENGINEER
EXPIRES 01-01-06

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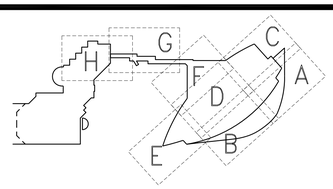
CONSTRUCTION SET
(REVISED-BID PACKAGE 6)

SPOKANE CONVENTION CENTER EXPANSION



REVISIONS

NO	DATE	BY	DESCRIPTION
5	10-20-04		BP-4 ADDENDUM #1
6	10-28-04		BP-4 ADDENDUM #2
7	11-04-04		BP-4 ADDENDUM #3
8			BP-4 CLARIFICATION DWG
9	01-21-05		BP-5
10	3-07-05		BP-6
11	4-15-05		CCD 025

DESIGNED	MA
DRAWN	JDC
CHECKED	JDC
DATE	4-15-05
CADD FILE	1505_0-01
JOB NUMBER	20150.00

TITLE
STRUCTURAL GENERAL NOTES

S0.1

8/4/2010 11:19:42 — XREF LIST: 150_title-CONS.DWG 150sno03.DWG 150sno02.DWG 150sno01.DWG 150sno04.DWG 150sno05.DWG 150sno06.DWG 150sno07.DWG 150sno08.DWG 150sno12.DWG 150sno14.DWG 150sno16.DWG 150sno21.DWG 150sno23.DWG 9sno15.DWG